

Proposed Residential Development, Former Marchon Works, Whitehaven

Flood Risk & Drainage Assessment

784-B030491

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1.0 INTRODUCTION

1.1 PURPOSE OF THIS REPORT

Tetra Tech Ltd have been appointed by Persimmon Homes Lancashire and NPL Ltd to prepare a Flood Risk and Drainage Assessment (FRDA) in support of a hybrid planning application for circa 540 units, commercial space and public open space.

Appendix A includes the proposed site layout.

1.2 PROPOSED DEVELOPMENT

The proposed development involves circa 540 residential units. The proposed hybrid application seeks detailed consent for 140 dwellings, and outline consent for circa 400 units and commercial space.

1.3 REQUIREMENT FOR A FLOOD RISK AND DRAINAGE ASSESSMENT

The application site is located in Flood Zone 1, (i.e. land assessed as having a less than 1 in 1,000 annual probability of river or sea flooding (<0.1% Annual Exceedance Probability (AEP)) in any one year). However, since the site has an area in excess of 1 ha, then in accordance with the National Planning Policy Framework (NPPF) and the Planning Practice Guide (Flood Risk & Coastal Change) (PPG), a Flood Risk Assessment is required to support the planning application.

The Parliamentary Written Statement¹ require applications for all major developments to be supported by a surface water strategy and where possible and reasonable, provide sustainable drainage systems (SuDS) in new developments, in order to protect people and property from flood risk and ensure that the flood risk to areas elsewhere is not increased.

1.4 SCOPE OF THE REPORT

The FRDA will be undertaken in accordance with the NPPF, Planning Practice Guidance (PPG) (Flood Risk and Coastal Change), Environment Agency guidance and Local Standards and Guidance for Surface Water Drainage in Cumbria. The FRDA will consider all potential sources of flood risk including Main Rivers, Ordinary Watercourses, overland flow routes, groundwater, sewers, reservoirs and canals. The FRDA will identify a surface water drainage strategy for the site such that flood risk to the site and adjoining areas is not exacerbated, as required by the NPPF, and there will be a particular focus on the provision of SuDS on the site.

¹ Department for Communities and Local Government (December 2014), *Sustainable drainage systems: Written statement - HCWS161*

The drainage assessment will also identify a foul drainage strategy. A review will be undertaken of the existing foul water drainage systems in the locality, to identify potential discharge points, and confirm the viability of any proposed connections to public sewerage infrastructure.

1.5 LIMITATIONS OF THIS REPORT

This report has been prepared by Tetra Tech Ltd on behalf of Persimmon Homes Lancashire and NPL Ltd in connection with the scope of the report as described above and takes into account the particular instructions and requirements set out in our fee proposal and the acceptance. It is not intended for and should not be relied on by any third party and no responsibility is undertaken to any third party.

Tetra Tech Ltd accepts no duty or responsibility (including in negligence) to any party other than Persimmon Homes Lancashire and NPL Ltd and disclaims all liability of any nature whatsoever to any such party in respect of this report.

This report cannot be reproduced without Tetra Tech's written consent.

2.0 SITE DESCRIPTION

2.1 EXISTING SITE

The site, as shown in Figure 1, is located within the eastern part of Whitehaven, on land previously occupied by the Marchon chemical works site. The site's postcode is CA28 9PE and the site's grid reference is NX 96539 16487.

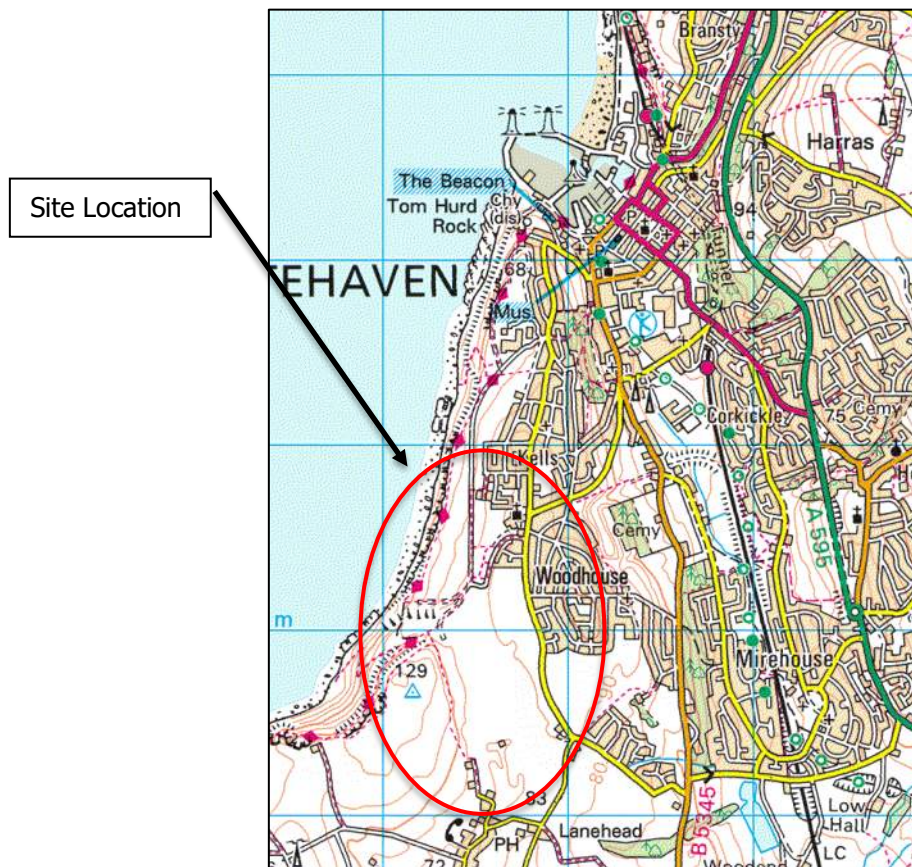


Figure 1 – Site Location²

The site covers an area of 28.34ha and comprises agricultural land on its northern part and some landscaping and abandoned roads on the former works site. It is currently defined as greenfield land (in terms of flood risk and planning policy). It is bounded to the north and east by residential developments, by the former works site to the south, and by landscaping and agricultural land to the west. Figure 2 shows the site boundary.

² © Crown copyright and database rights [2021] Ordnance Survey 0100031673

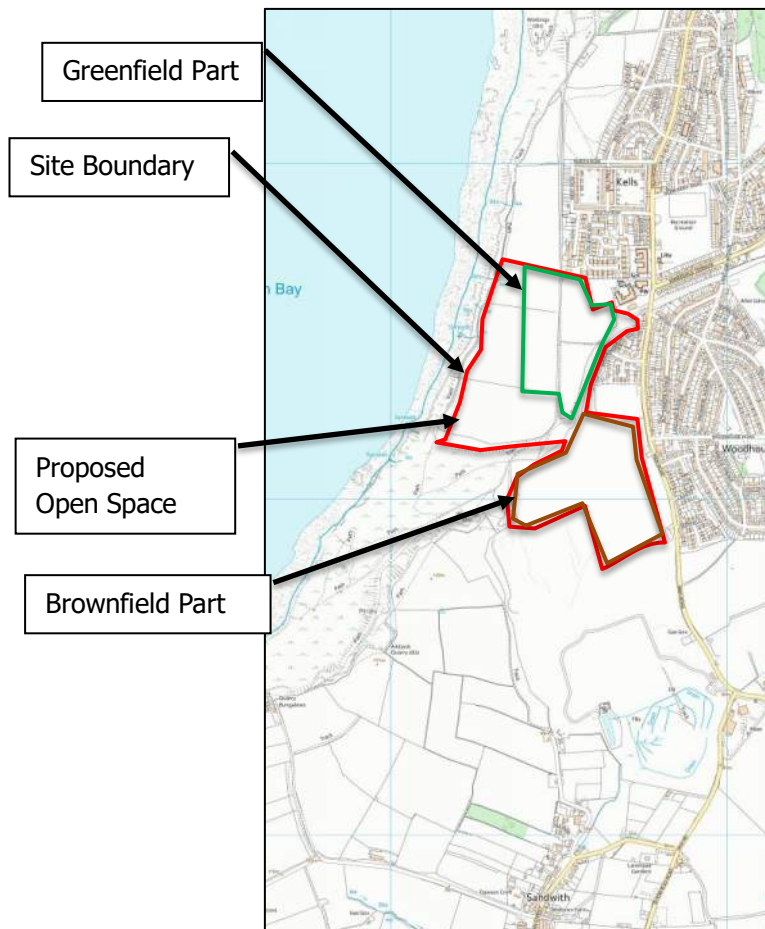


Figure 2 – Site Boundary Plan³

A review of the topographical survey indicates that the site generally falls from east to west. The highest level is approximately 97.3m AOD in the south eastern part of the site, and the lowest level is approximately 49.5m AOD in the most western part of the site.

The topographical survey can be found in Appendix B.

2.2 EXISTING DRAINAGE

2.2.1 Watercourses

There are no watercourses within or near the site.

³ © Crown copyright and database rights [2021] Ordnance Survey 0100031673

2.2.2 Drains and Sewers

The United Utilities (UU) records show the presence of the following public sewers:

- 225mm foul sewers flowing north through the central part of the site.
- 375mm surface water sewer in Hartfield Close.
- 150mm foul water sewer in Hartfield Close.
- 150mm foul sewer flowing west through the site from the Waters Edge Close development site.
- Surface water sewer and attenuation tank flowing west through the northern part of the site serving the Waters Edge Close development site.

In addition to the UU sewers, there are two private surface water drains to the eastern part of the site, that discharges into the sea and served the former chemical works site. The existing drains and outfalls were built to serve the former factory. The northern pipe comprises a 600mm diameter steel pipe, with the southern pipe being a 450mm diameter pipe.

Previous surveys by others show that there is a 150mm diameter plastic pipe flowing through the 600mm steel pipe, discharging leachate from the old mine. It is anticipated this discharge will cease once the site is remediated and developed, however this is to be confirmed by the specialist consultant.

Existing Outfalls

Tetra Tech visited site to determine the existing condition of the existing outfalls. The northern outfall discharges into the sea via a submerged outfall. There is a shaft chamber in the rock that enables the pipe to drop to beach level. The second outfall is located at high level, with runoff presumably cascading down the cliff onto the beach.

The sewer records by UU are included in Appendix C. Appendix D includes the survey of the existing private drains by others. Appendix E includes relevant photographs from Tetra Tech's visit.

2.3 GROUND CONDITIONS

2.3.1 Geology

Borehole records available in the BGS website for a location in close proximity to the site indicate that the superficial deposits comprise clays, then underlain by sandstone.

It is anticipated that there is made ground within the former chemical works site. The made ground is likely to include contaminants which will be remediated prior to development.

2.3.2 Hydrogeology

The site is not situated within a groundwater Source Protection Zone (SPZ) or a drinking water safeguard zone (groundwater).

3.0 FLOOD RISK

3.1 FLUVIAL AND COASTAL FLOOD RISK

Fluvial flood risk is the risk arising from Main Rivers and Ordinary Watercourses. Coastal flood risk is the risk of flooding from the sea (tidal surges, waves, sea level raise). A floodplain is the area that would naturally be affected by flooding if a river rises above its banks. In England, floodplains of Main Rivers are divided into flood zones for planning purposes. These show the extent of the natural floodplain area at risk of inundation if there were no flood defences or certain other manmade structures and channel improvements. They are divided as follows:

- Flood Zone 3 shows the land having a 1 in 100 or greater annual probability of river flooding; or a 1 in 200 or greater annual probability of flooding from the sea.
- Flood Zone 2 shows the additional extent of an extreme flood from rivers. It is land having between a 1 in 100 and 1 in 1,000 annual probability of river flooding.
- Flood Zone 1 is the area of land where flooding from rivers and the sea is very unlikely.

The Environment Agency's Flood Map for Planning indicates that the site is located entirely within Flood Zone 1 and therefore at very low risk of fluvial flooding from Main Rivers. This is shown in Figure 3 below. Whilst the site is near the sea, land within the site is raised to significant height above sea level with cliffs running along the shoreline, sea level rise would not impact on the site. There are no proposals to develop land adjacent the cliffs where coastal erosion could occur.

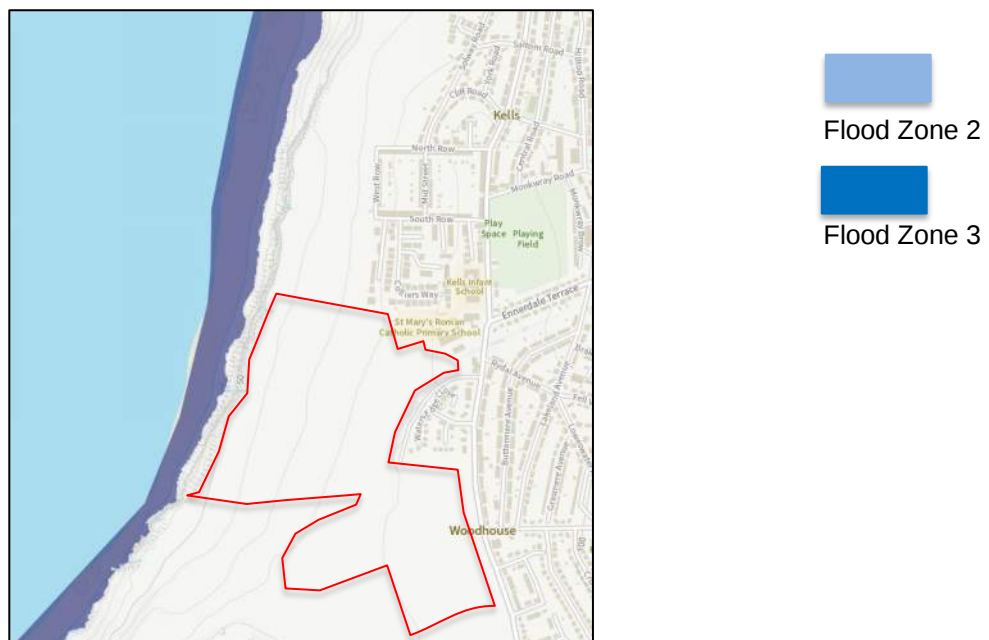


Figure 3 - Environment Agency Flood Map for Planning (June 2021)

3.2 SURFACE WATER AND OVERLAND FLOWS

Surface water flooding occurs where high rainfall events exceed the drainage capacity in an area (i.e. sewer system and/or watercourse), leading to flooding.

The Environment Agency's Flood Risk from Surface Water Map shown as Figure 4 indicates that the site is at very low risk of flooding from surface water. The map shows ponding within a few areas in the site, however these are likely associated with low lying areas that will be removed when the site is developed.



Figure 4 – Environment Agency Surface Water Flood Risk Map (June 2021)

3.3 GROUNDWATER FLOODING

Groundwater flooding occurs when water levels in the ground rise above the land surface. This type of flooding is most likely to occur in areas above an aquifer.

The Copeland Borough Council Strategic Flood Risk Assessment (SFRA) states that some areas in Whitehaven have experienced groundwater flooding, however these are not near the site. Due to the Clay based ground conditions nearer the surface, it is considered that the site is at low risk of flooding from groundwater.

3.4 SEWER FLOODING

Sewer flooding occurs when intense rainfall overloads the sewer system capacity and/or when sewers cannot discharge properly to watercourses due to high water levels. Sewer flooding can also be caused when problems such as blockages, collapses or equipment failure occur in the sewerage system.

There are a number of sewers within the site, however these are recent and have been designed in accordance with modern standards, and are subject to regular maintenance by UU. Therefore, it is considered that the risk of flooding from sewers to the site is very low.

3.5 RESERVOIR FLOODING

Although the probability of a catastrophic dam failure is considered to be extremely low, the consequence of such an event would be severe. The Environment Agency's Flood Risk from Reservoirs Map indicates that the site is not at risk of flooding as a result of reservoir failure.

3.6 SUMMARY OF FLOOD RISK

Overall, the flood risk to the site from all sources is low.

It will be essential to ensure that no increase in flood risk occurs downstream of the site or on adjacent areas as a result of this development. The proposed mitigation measures are discussed in more detail within Section 4.

4.0 DEVELOPMENT PROPOSALS

4.1 PROPOSED DEVELOPMENT

A hybrid planning application is being submitted to Copeland BC for the development of circa 540 dwellings within the site. The detailed part of the hybrid application covers 140 dwellings, with the remaining 400 dwellings and commercial space seeking outline approval.

Appendix A includes the proposed site layout.

4.2 DEVELOPMENT AND FLOOD RISK

4.2.1 Flood Risk to the Development

As assessed in Section 3, the site is considered to be at low risk of flooding from all sources.

4.2.2 Flood Risk Arising from the Development

As an existing greenfield site in flood risk terms, the proposed development will increase the impermeable area of the site. This will generate additional surface water runoff which, if uncontrolled, could increase the risk of surface water flooding elsewhere. It is therefore necessary to manage surface water runoff on site to avoid increasing the flood risk elsewhere.

4.3 SURFACE WATER MANAGEMENT

4.3.1 Drainage Hierarchy

In order to ensure that surface water runoff from the site does not increase the risk of flooding, the management of runoff should be considered via a sequential approach, in line with Building Regulations⁴. The following options for the disposal of runoff have been considered, in order of preference:

- Discharge to an adequate soakaway or some other adequate infiltration system (i.e. to ground);
- Discharge to a watercourse; and
- Discharge to a sewer (surface water before combined).

Discharge to Ground

Given the site's underlying geology and expected presence of contaminants within the ground, infiltration is not considered to be feasible.

⁴ HM Government (2010), *The Building Regulations 2010, Approved Document H (Part H3)*

Discharge to Watercourse

This is not considered a practical alternative as there are no watercourses near the site.

Discharge to Sewer

There is an existing surface water sewer within the site that used to serve the former factory and now discharges to the sea. It is anticipated that a new connection to this drain will be feasible and it is proposed to discharge surface water runoff from this site using the existing outfall.

4.3.2 Pre and Post-Development Areas

The existing site is entirely greenfield in flood risk terms, therefore 100% permeable. The post-development areas have been calculated using the proposed site layout, found in Appendix A.

The pre and post-development areas are shown in Table 1 below.

Table 1 – Pre and Post-Development Areas (ha)

Status	Impermeable Area	Permeable Area
Pre-development	0	28.34
Post-development	10.89	17.45

4.3.3 Surface Water Discharge Rates

The site is greenfield in flood risk terms. Table 2 below shows the greenfield runoff rates for the site, that have been estimated using the UK SuDS online tool.

It is proposed to discharge surface water runoff from the proposed development site to the sea. It is not therefore necessary to restrict discharge rates. The anticipated discharge rates would only be restricted by the capacity of the existing pipes and these will be confirmed at the detailed design stage.

Table 2 – Greenfield Runoff Rates (l/s)

Qbar (l/s/ha)	1 in 1 year	1 in 30 year	1 in 100 year	1 in 100 year + 40% CC
7.8	193	377.1	461.4	N/A

Appendix F includes the greenfield runoff calculations.

4.3.4 Proposed Surface Water Drainage

As noted above, it is proposed to discharge surface water from the site to the sea via the existing outfalls. There are ongoing discussions with UU to determine the future adoption of the existing structures, however at this stage it is assumed that at least one of the existing outfalls can be retained. Given that a hybrid planning application is being submitted, covering different parts of the site, the strategies for each phase are explained below.

a) Greenfield Site

The detailed part of the application seeks permission for 139 dwellings and associated infrastructure at the northern part of the site. Levels in this part of the site fall to the west, however a gravity connection to the existing pipe is considered to be feasible. A main carrier drain will be provided in the proposed access road alongside the eastern edge of the proposed residential development. This pipe will then connect to the existing network at one of the existing manholes.

b) Brownfield Site

The site falls from south east to north west. A series of main carrier drains will be provided in order to convey flows towards the existing drain to be retained.

The proposed drainage systems for both parts of the site are to be designed in line with the requirements of Appendix C of the Sewerage Sector Guidance (SSG).

An assessment of the applicability of the different SuDS features is included in Section 5 below. Appendix G includes a preliminary drainage layout.

4.3.5 Required Attenuation

Given the nature of the contours within the site, it is anticipated that significant exceedance flows would not be able to be retained within external areas, and therefore in order to minimise any flooding of the site, in accordance with national and local policy, it is proposed that the new drainage network will be designed so that there is no flooding of surface water runoff on site up to and including the 1 in 30 year event in line with the SSG's requirements. Exceedance flows above this event can be routed towards the beach through the roads and access roads.

It is anticipated that some attenuation may be required given the impermeable area to be drained through the existing outfalls. The attenuation could be provided within oversized pipes and manholes, this is to be determined at the detailed design stage.

4.4 RESIDUAL FLOOD RISK AND EXCEEDANCE ROUTES

The primary residual risk would be associated with a failure of the surface water drainage system. The risk of flooding would also increase if the drainage system is not maintained on a regular, ongoing basis.

In addition, there remains a residual risk of a storm event that exceeds the capacity of the drainage system, as events beyond the 1 in 100 year storm event plus 40% allowance for climate change storm will not be catered for explicitly.

The existing fall will be maintained, with exceedance flow routes following the contours of the land to the west.

5.0 SUSTAINABLE DRAINAGE

5.1 REVIEW OF SUDS OPTIONS

In order to comply with the national guidelines and policies set by the NPPF and the Non-Statutory Technical Standards for Sustainable Drainage, as well as those of HCC, the design of the surface water drainage system should seek to maximise the use of SuDS techniques. This section reviews the suitability of the different SuDS features available for the proposed development site.

5.2 THE SUDS MANAGEMENT TRAIN

The main purpose of SuDS is to manage the surface water runoff generated by a development within the development site, attenuating additional flows generated by the introduction of impermeable areas whilst providing water quality treatment to the runoff and amenity and landscape benefits to the community. SuDS features can be categorised as follows:

- Source Control: manage runoff at its source
 - Water butts, green/brown roofs, permeable pavements, rainwater harvesting systems, bio-retention systems.
- Site Control: manage runoff generated by a wider area
 - Swales, ponds, infiltration devices, filter strip, French drains.
- Regional Control: manage runoff generated by several sites
 - Basins, ponds and wetlands

The following is an illustration of the SuDS principles and how they may be applied to a development via a SuDS Management Train.

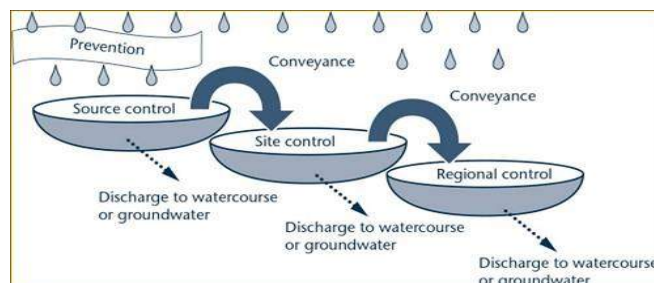


Figure 4 – SuDS Management Train

Table 3 below summarises the suitability of the different SuDS elements for the proposed development.

Table 3 – Review of SuDS elements for the proposed development

Type of SuDS	Description	Applicability to the Site
Source Control		
Water butts	Small storage tanks on each individual housing plot.	This would be appropriate on individual dwellings.
Rainwater harvesting	Recycling of water from roofs and impermeable areas.	This would be appropriate on the proposed commercial units.
Green roofs	Vegetated roofs that reduce runoff and remove pollutants.	Green roofs are unlikely to be appropriate for individual dwellings.
Pervious surfaces	Pavements that allow surface water to flow into underlying layers of the pavement and either infiltrate or drain to an on-site drainage network.	This may be appropriate within the proposed private drives.
Rain Gardens & Bioretention Systems	Shallow depressions with free draining soil and planted with vegetation that withstands occasional flooding.	This could be incorporated into landscaped areas.
Site & Regional Control		
Filter Drains	Linear drains or trenches filled with granular material that allow infiltration to the surrounding ground.	This is appropriate to serve the road alongside the western edge of the greenfield site.
Swales	Vegetated channels to convey store and treat runoff.	These could be provided within areas of public open space.
Detention basins & ponds	Shallow areas of open space that temporarily hold water and collect silt.	The existing contours limit the practicality of detention basins within this site.
Infiltration basins	Shallow depression that stores runoff before it infiltrates into the subsoil.	These are unlikely to be appropriate within this site.
Infiltration devices	Generally granular trenches or soakaways that store water and allow infiltration to the surrounding ground.	

Table 3 identifies that there are several SUDS components that could be used within the development site, subject to detailed design. Above ground attenuation is preferred to below ground attenuation, for

reasons including biodiversity, amenity benefits and maintainability. There is scope to incorporate above ground attenuation through the use of filter drains and infiltration swales to drain roads and parking areas.

All SuDS are to be designed in accordance with the The SuDS Manual and the relevant British Standards.

5.3 WATER QUALITY

In accordance with Table 26.2 of The SuDS Manual CIRIA C753, the pollution hazard level is considered to be 'Low' for the proposed land use. Therefore, the requirements for discharge to surface waters state that the 'Simple index approach' should be used. Step 1 of the simple index approach is to identify the pollution hazard indices for the proposed land use as set out in Table 4 below, which is an extract of Table 26.2.

Step 1 of the simple index approach is to identify the pollution hazard indices for the proposed land use. Table 26.2 of The SuDS Manual states the following:

Table 4 - Pollution Hazard Indices

Land Use	Pollution Hazard Level	Total Suspended Solids (TSS)	Metals	Hydrocarbons
Residential roofs	Very low	0.2	0.2	0.05
Individual property driveways, low traffic roads	Low	0.5	0.4	0.4
most roads	Medium	0.7	0.6	0.7

Table 26.4 of The SuDS Manual states the various mitigation indices for different SuDS elements. These are shown below in Table 5.

Table 5 – SuDS Mitigation Indices (Discharge to Surface Water)

SuDS Feature	TSS	Metals	Hydrocarbons
Filter drain	0.4	0.4	0.4
Swale	0.5	0.6	0.6
Bioretention system	0.8	0.8	0.8

Provided that the mitigation indices of the treatment techniques are greater than or equal to the hazard indices for the proposed development then there should be no reduction in the overall water quality within the receiving system. A combination of swales or a treatment device would provide sufficient treatment to the runoff before being discharged to the sea.

If multiple SuDS features are used in combination, the total SuDS mitigation index for each pollutant is a combination of the mitigation indices of each element. The first SuDS feature of the train will always

be more effective than the subsequent features, given that the concentration of pollutants in the runoff entering these is lower.

It should be noted that alternative SuDS options may also be considered during the detailed design stage which achieves or exceeds the water quality objective. Provided that the mitigation indices of the treatment techniques are greater than or equal to the hazard indices for the proposed development then there should be no reduction in the overall water quality within the receiving system.

5.4 DRAINAGE AND SUDS MAINTENANCE

SuDS require regular maintenance to keep them working effectively.

The proposed drainage system is to be offered to UU for adoption, and there are ongoing discussions with UU to determine their requirements to adopt the existing outfall structures. Table 6 below summarises the anticipated maintenance requirements.

Table 6 – Maintenance tasks and frequency required

SuDS Feature	Maintenance Task	Recommended Frequency
Oversized Pipes	- Remove debris from catchment surface - Remove sediment from pre-treatment structures - Inspect inlets and outlets - Survey inside of the pipes	- Every 12 months - Every 12 months - Every 12 months - Every 5 years
Swale	- Inspect inlets and outlets for blockages. Clear if required - Remove litter and dead leaves - Manage vegetation and remove nuisance plants - Grass cutting and vegetation management. Minimum grass length 75mm, max grass length 200mm - Inspect inlets and outlets and swales for silt accumulation and establish a silt removal frequency.	- Monthly - Monthly (as required) - Monthly (as required) - As required - Every 6 months

6.0 FOUL DRAINAGE

6.1 EXISTING FOUL DRAINAGE

There is an existing 225mm diameter foul sewer flowing north through the central part of the site.

6.2 PROPOSED FOUL DRAINAGE

The development proposals include up to 540 units. In accordance with the Sewerage Sector Guidance, the expected peak foul flow from this number of units is 27 l/s. United Utilities is reviewing the existing capacity in the sewer to confirm if any upgrades would be required.

As a result of the existing contours, it is anticipated that the western half of the dwellings proposed in the northern part of the site would not be able to drain by gravity to the existing sewer. Therefore, a pumping station will be required to convey the flows to the UU foul sewer. A final review of levels in the brownfield part of the site will be required to determine whether a gravity discharge to the existing sewer would be practical.

7.0 CONCLUSIONS AND RECOMMENDATIONS

7.1 CONCLUSIONS

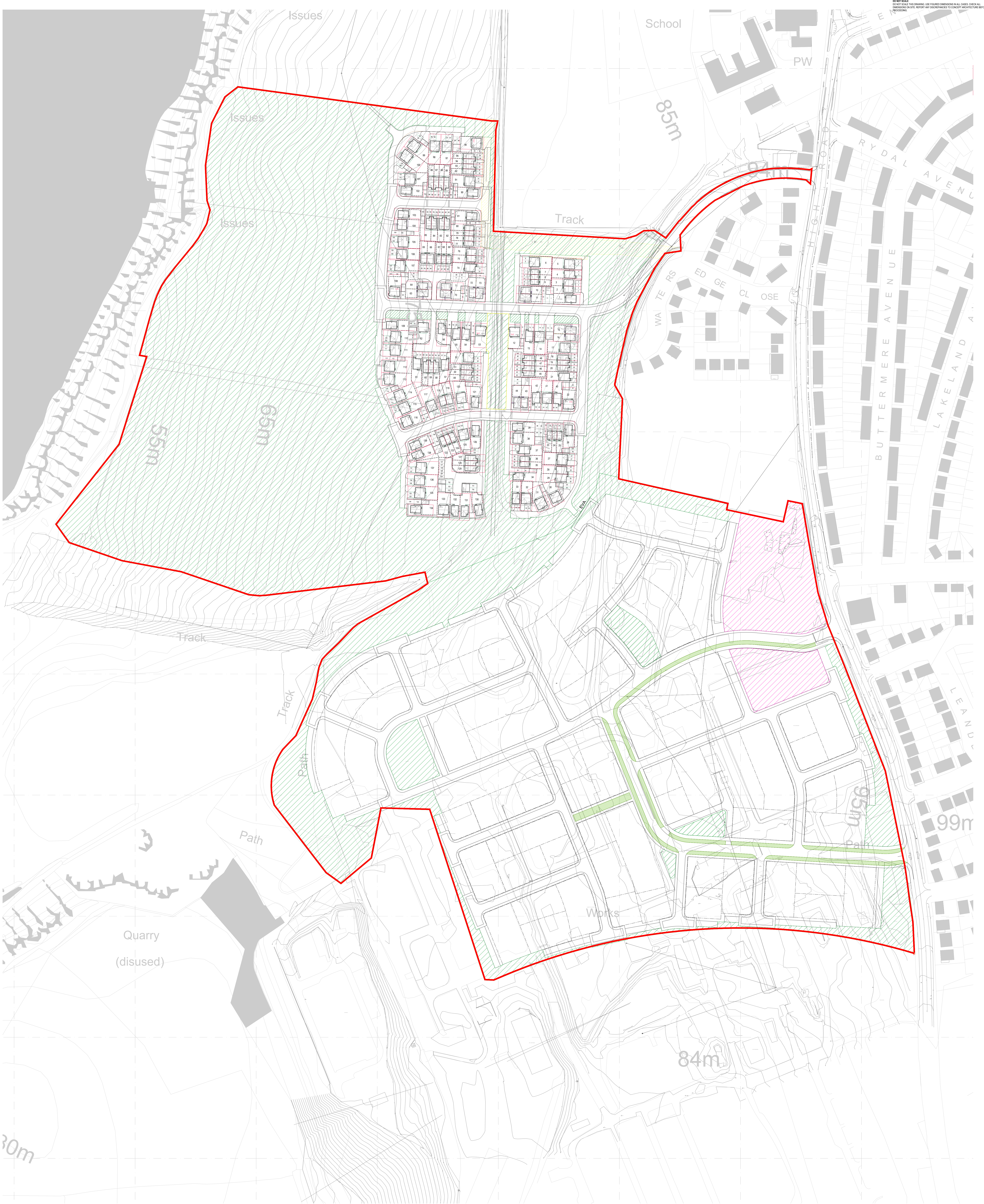
1. This FRDA has been produced in support of a hybrid planning application for the development of circa 540 dwellings in the former Marchon site in Whitehaven.
2. The FRDA has been undertaken in accordance with the NPPF (Chapter 14), PPG (Flood Risk and Coastal Change), Environment Agency guidance and Guidance for Surface Water Drainage in Cumbria.
3. This assessment has identified that the proposed development can be implemented in accordance with the relevant flood risk and drainage national and local planning policies.
4. The application site area covers 28.34 ha and falls within Flood Zone 1.
5. The site is at low risk of flooding from all sources.
6. Surface water runoff from the site is to be drained to the sea via two existing outfalls serving the former factory's drainage system.
7. Runoff rates are not required to be restricted, however they will be limited by the capacity of the existing outfalls.
8. The proposed drainage system will be offered to UU for adoption, including the existing outfalls.
9. It is proposed to drain foul flows to the existing UU public foul sewer.

7.2 RECOMMENDATIONS

1. Further liaison with UU is required to confirm the future maintenance of the existing outfalls.
2. Additional surveys of the existing drains are required to confirm the condition and any required improvements.
3. Final levels on the brownfield part of the site could be designed to enable a gravity connection to the existing foul public sewer.
4. All SUDS should be designed in accordance with The SUDS Manual (Ciria C753) and Defra's Non-technical standards for sustainable drainage systems.
5. A comprehensive inspection and maintenance schedule shall be provided and implemented to ensure that the drainage system is maintained in a good operational manner.

APPENDICES

APPENDIX A – PROPOSED SITE LAYOUT



BLOCK PLAN KEY

- PUBLIC OPEN SPACE - 12.0117 Ha = 29.7310 Acres
- BLOCK STRUCTURE
- COMMERCIAL AREA - 2.41 Acres
- PROPOSED GROSS SITE BOUNDARY - 70.5132 Acres
- NETT DEVELOPABLE AREA - 34.5241 Acres

PHASING PLAN KEY

Phase 1

- Orange Line: 14.0715 Ha = 34.7714 Acres
- Nett Area: 3.7814 Ha X 2.471 = 9.3441 Acres = 139 units

Phase 2

- Purple Line: 14.4642 Ha = 35.7419 Acres
- Nett Area: 10.1948 Ha X 2.471 = 25.1842 Acres = 377 units

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FOR PERMISON HOMES LANCASHIRE			
drawing title			
PROPOSED MASTERPLAN PLAN			
project no.	drawing no.	revision	date
CPT-257	MP_01	-	08.03.21
scale	drawn by	sheet size	
1/1000	KB	A0	
320J Mayoral Way Team Valley Trading Estate Gateshead NE11 0PT			
0191 - 4661613 info@cpt-group.co.uk			

APPENDIX B – TOPOGRAPHICAL SURVEY

APPENDIX C – PUBLIC SEWER RECORDS

Francisco Aguilar

**3 Sovereign Square
Sovereign Street, West Yorkshire
Leeds,
LS1 4ER**

FAO:

How to contact us:

**United Utilities Water Limited
Property Searches
Haweswater House
Lingley Mere Business Park
Great Sankey
Warrington
WA5 3LP**

Telephone: 0370 7510101

E-mail: propertysearches@uuplc.co.uk

**Your Ref: B030491
Our Ref: UUPS-ORD-302870
Date: 01/07/2021**

Dear Sirs

Location: 1 WATERS EDGE CLOSE, WHITEHAVEN, CA28 9PE

I acknowledge with thanks your request dated 30/06/2021 for information on the location of our services.

Please find enclosed plans showing the approximate position of United Utilities' apparatus known to be in the vicinity of this site.

The enclosed plans are being provided to you subject to the United Utilities terms and conditions for both the wastewater and water distribution plans which are shown attached.

If you are planning works anywhere in the North West, please read United Utilities' access statement before you start work to check how it will affect our network. <http://www.unitedutilities.com/work-near-asset.aspx>.

I trust the above meets with your requirements and look forward to hearing from you should you need anything further.

If you have any queries regarding this matter please [contact us](#).

Yours Faithfully,



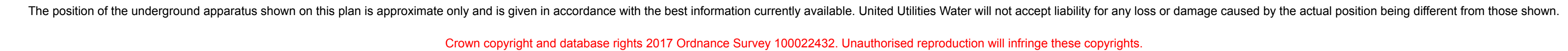
Karen McCormack
Property Searches Manager

TERMS AND CONDITIONS - WASTEWATER AND WATER DISTRIBUTION PLANS

These provisions apply to the public sewerage, water distribution and telemetry systems (including sewers which are the subject of an agreement under Section 104 of the Water Industry Act 1991 and mains installed in accordance with the agreement for the self construction of water mains) (UUWL apparatus) of United Utilities Water Limited "(UUWL)".

TERMS AND CONDITIONS:

- This Map and any information supplied with it is issued subject to the provisions contained below, to the exclusion of all others and no party relies upon any representation, warranty, collateral contract or other assurance of any person (whether party to this agreement or not) that is not set out in this agreement or the documents referred to in it.
- This Map and any information supplied with it is provided for general guidance only and no representation, undertaking or warranty as to its accuracy, completeness or being up to date is given or implied.
- In particular, the position and depth of any UUWL apparatus shown on the Map are approximate only. UUWL strongly recommends that a comprehensive survey is undertaken in addition to reviewing this Map to determine and ensure the precise location of any UUWL apparatus. The exact location, positions and depths should be obtained by excavation trial holes.
- The location and position of private drains, private sewers and service pipes to properties are not normally shown on this Map but their presence must be anticipated and accounted for and you are strongly advised to carry out your own further enquiries and investigations in order to locate the same.
- The position and depth of UUWL apparatus is subject to change and therefore this Map is issued subject to any removal or change in location of the same. The onus is entirely upon you to confirm whether any changes to the Map have been made subsequent to issue and prior to any works being carried out.
- This Map and any information shown on it or provided with it must not be relied upon in the event of any development, construction or other works (including but not limited to any excavations) in the vicinity of UUWL apparatus or for the purpose of determining the suitability of a point of connection to the sewerage or other distribution systems.
- No person or legal entity, including any company shall be relieved from any liability howsoever and whensoever arising for any damage caused to UUWL apparatus by reason of the actual position and/or depths of UUWL apparatus being different from those shown on the Map and any information supplied with it.
- If any provision contained herein is or becomes legally invalid or unenforceable, it will be taken to be severed from the remaining provisions which shall be unaffected and continue in full force and affect.
- This agreement shall be governed by English law and all parties submit to the exclusive jurisdiction of the English courts, save that nothing will prevent UUWL from bringing proceedings in any other competent jurisdiction, whether concurrently or otherwise.



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1 WATERS EDGE CLOSE,
WHITEHAVEN,
CA28 9PE

**SEWER
RECORDS**

 **United
Utilities**
Water for the North West

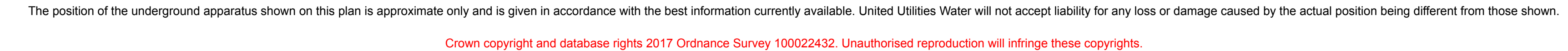


Figure 1 is a diagram illustrating the spatial distribution of various water quality parameters across different locations. The locations are categorized into four main groups: Abandoned, Foul, Surface Water, and Combined. The parameters are represented by colored dots and lines, indicating their presence or concentration at specific locations. The parameters include Public Sewer, Private Sewer, Section 104, Rising Main, Sludge Main, Overflow, Water Course, and Highway Drain. The diagram shows that certain parameters are concentrated in specific locations, such as Public Sewer and Private Sewer in the Abandoned group, and Water Course and Highway Drain in the Surface Water group.

Manhole	Side Entry Manhole
Head of System	Outfall
Extent of Survey	Screen Chamber
Rodding Eye	Inspection Chamber
Inlet	Bifurcation Chamber
Discharge Point	Lamp Hole
Vortex	T Junction / Saddle
Penstock	Catchpit
Washout Chamber	Valve Chamber
Valve	Vent Column
Air Valve	Vortex Chamber
Non Return Valve	Penstock Chamber
Soakaway	Network Storage Tank
Gully	Sewer Overflow
Cascade	Ww Treatment Works
Flow Meter	Ww Pumping Station
Hatch Box	Septic Tank
Oil Interceptor	Control Kiosk
Summit	Change of Characteristic
Drop Shaft	
Orifice Plate	

FO	Foul
SW	Surface Water
CO	Combined
OV	Overflow

CI	Circular	TR	Trapezoidal
EG	Egg	AR	Arch
OV	Oval	BA	Barrel
FT	Flat Top	HO	HorseShoe
RE	Rectangular	UN	Unspecified
SQ	Square		

AC	Asbestos Cement
BR	Brick
PE	Polyethylene
RP	Reinforced Plastic Matrix
CO	Concrete
CSB	Concrete Segment Bolted
CSU	Concrete Segment Unbolted
CC	Concrete Box Culverted
PSC	Plastic / Steel Composite
GRC	Glass Reinforced Plastic
DI	Ductile Iron
PVC	Polyvinyl Chloride
CI	Cast Iron
SI	Spun Iron
ST	Steel
VC	Vitritified Clay
PP	Polypropylene
PF	Pitch Fibre
MAC	Masonry, Coursed
MAR	Masonry, Random
U	Unspecified

1 WATERS EDGE CLOSE,
WHITEHAVEN,
CA28 9PE

Printed by: Property Searches

**SEWER
RECORDS**

 **United
Utilities**
Water for the North West

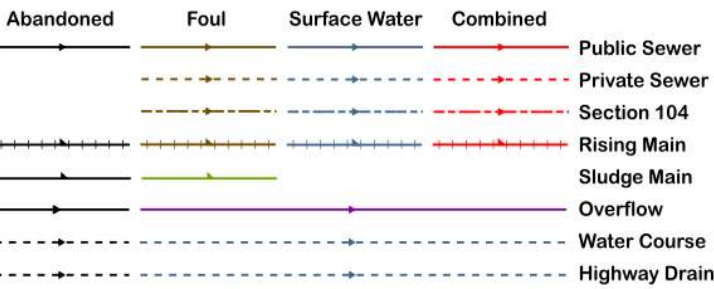


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Ratio	Color	Func	Conn	Inst	Size	z	Size	y	Shape	Matt	Length	Gr	Int
5201	66.37				68.48		68.48				45	70.0837	1 n 127
5202	65.92				66.42		66.42				92	82.20087	1 n 98
5203	65.92				66.42		66.42				92	82.20087	1 n 98
5204	68.97				70.065		70.065				60	60.00533	1 n 138
5205	72.2				70.065		70.065				60	60.00533	1 n 138
5910	72.2				70.065		70.065			VC	4		
5911	72.2				70.065		70.065			VC	4		
5912	72.2				70.065		70.065			VC	4		
5913	72.2				70.065		70.065			VC	4		
5905	69.65				67.89		67.89				103	103.049	1 n 94
5906	69.65				67.89		67.89				103	103.049	1 n 94
5907	69.65				67.89		67.89				103	103.049	1 n 94
5908	69.65				67.89		67.89				103	103.049	1 n 94
5909	69.65				67.89		67.89				103	103.049	1 n 94
5901	71.05				68.84		68.84			VC	1	1 n 94	
5902	71.05				68.84		68.84			VC	1	1 n 94	
5903	71.05				68.84		68.84			VC	1	1 n 94	
5904	71.05				68.84		68.84			VC	1	1 n 94	
5905	71.05				68.84		68.84			VC	1	131.0001	1 n 9
5906	66.12				65.98		65.98			VC	131	131.0001	1 n 9
5907	66.12				65.98		65.98			VC	75	75.0159	1 n 75
5908	66.12				65.98		65.98			VC	75	75.0159	1 n 75
5909	66.12				65.98		65.98			VC	75	75.0159	1 n 75
5910	76.48				68.97		68.97				83	83.07073	1 n 75
5911	76.48				68.97		68.97				83	83.07073	1 n 75

Ratio	Cover	Func	Invert	Size x	Size y	Shape	Mat	Length	Grad
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LEGEND



All point assets follow the standard colour convention:

red - combined blue - surface water
brown - foul purple - overflow

- Manhole
 - Head of System
 - Extent of Survey
 - Rodding Eye
 - Inlet
 - Discharge Point
 - Vortex
 - Penstock
 - Washout Chamber
 - Valve
 - Air Valve
 - Non Return Valve
 - Soakaway
 - Gully
 - Cascade
 - Flow Meter
 - Hatch Box
 - Oil Interceptor
 - Summit
 - Drop Shaft
 - Orifice Plate
 - Side Entry Manhole
 - Outfall
 - Screen Chamber
 - Inspection Chamber
 - Bifurcation Chamber
 - Lamp Hole
 - T Junction / Saddle
 - Catchpit
 - Valve Chamber
 - Vent Column
 - Vortex Chamber
 - Penstock Chamber
 - Network Storage Tank
 - Sewer Overflow
 - Ww Treatment Works
 - Ww Pumping Station
 - Septic Tank
 - Control Kiosk
- Change of Characteristic

MANHOLE FUNCTION

FO	Foul
SW	Surface Water
CO	Combined
OV	Overflow

SEWER SHAPE

CI	Circular	TR	Trapezoidal
EG	Egg	AR	Arch
OV	Oval	BA	Barrel
FT	Flat Top	HO	HorseShoe
RE	Rectangular	UN	Unspecified
SQ	Square		

SEWER MATERIAL

AC	Asbestos Cement
BR	Brick
PE	Polyethylene
RP	Reinforced Plastic Matrix
CO	Concrete
CSB	Concrete Segment Bolted
CSU	Concrete Segment Unbolted
CC	Concrete Box Culverted
PSC	Plastic / Steel Composite
GRC	Glass Reinforced Plastic
DI	Ductile Iron
PVC	Polyvinyl Chloride
CI	Cast Iron
SI	Spun Iron
ST	Steel
VC	Vitrified Clay
PP	Polypropylene
PF	Pitch Fibre
MAC	Masonry, Coursed
MAR	Masonry, Random
U	Unspecified

Address or Site Reference:

1 WATERS EDGE CLOSE,
WHITEHAVEN,
CA28 9PE

OS sheet NX9617SW
Number:

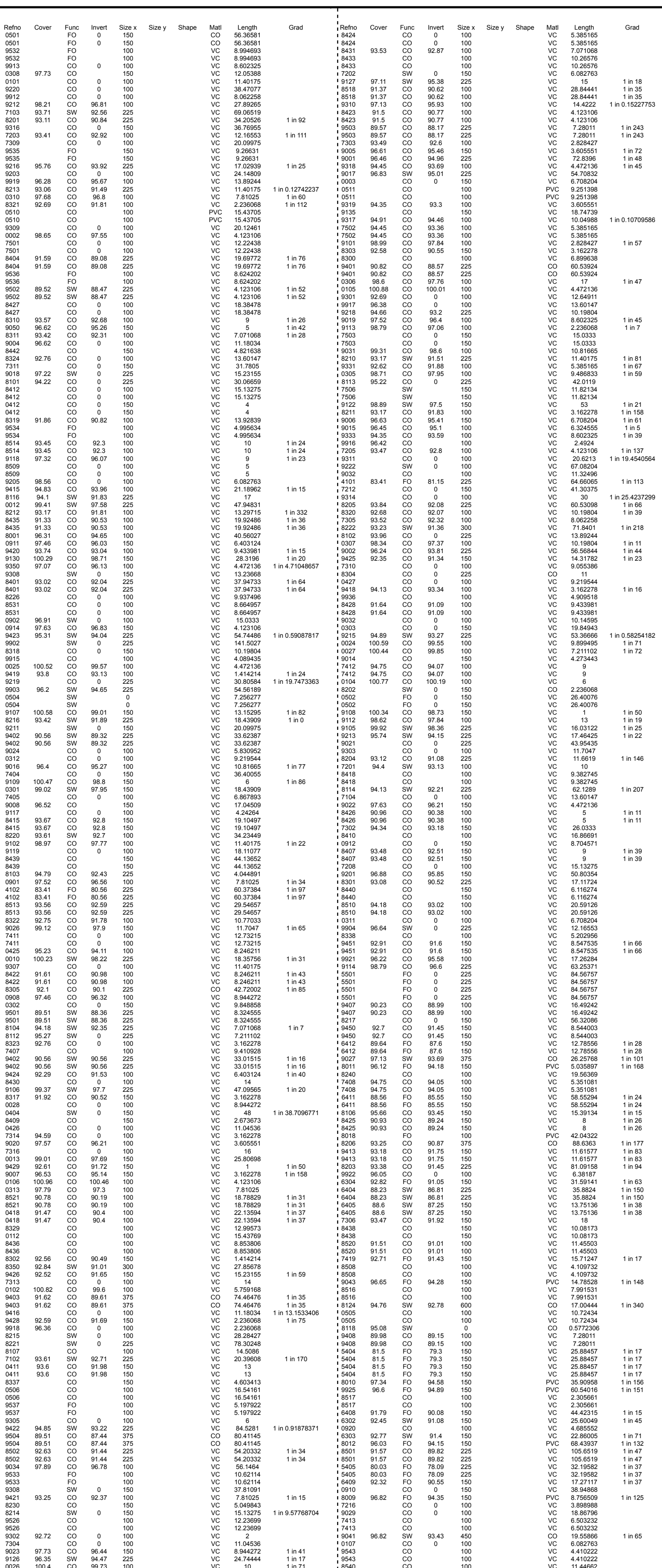
Scale: 1:1250 **Date:** 01/07/2021

Nodes: 24
Sheet: 3 of 6

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SEWER RECORDS





LEGEND

Abandoned
Foul
Surface Water
Combined

All point assets follow the standard colour convention:
red - combined blue - surface water
brown - foul purple - overflow

● Manhole ⦿ Head of System ⦿ Extent of Survey ⦿ Rodding Eye ⦿ Inlet ⦿ Discharge Point ⦿ Vortex ⦿ Penstock ⦿ Washout Chamber ⦿ Valve ⦿ Air Valve ⦿ Non Return Valve ⦿ Soakaway ⦿ Gully ⦿ Cascade ⦿ Flow Meter ⦿ Hatch Box ⦿ Oil Interceptor ⦿ Summit ⦿ Drop Shaft ⦿ Orifice Plate	⦿ Side Entry Manhole ⦿ Outfall ⦿ Screen Chamber ⦿ Inspection Chamber ⦿ Bifurcation Chamber ⦿ Lamp Hole ⦿ T Junction / Saddle ⦿ Catchpit ⦿ Valve Chamber ⦿ Vent Column ⦿ Vortex Chamber ⦿ Penstock Chamber ⦿ Network Storage Tank ⦿ Sewer Overflow ⦿ Ww Treatment Works ⦿ Ww Pumping Station ⦿ Septic Tank ⦿ Control Kiosk ▽ Change of Characteristic
---	---

MANHOLE FUNCTION

FO	Foul
SW	Surface Water
CO	Combined
OV	Overflow

SEWER SHAPE

CI	Circular	TR	Trapezoidal
EG	Egg	AR	Arch
OV	Oval	BA	Barrel
FT	Flat Top	HO	HorseShoe
RE	Rectangular	UN	Unspecified
SQ	Square		

SEWER MATERIAL

AC	Asbestos Cement
BR	Brick
PE	Polyethylene
RP	Reinforced Plastic Matrix
CO	Concrete
CSB	Concrete Segment Bolted
CSU	Concrete Segment Unbolted
CC	Concrete Box Culverted
PSC	Plastic / Steel Composite
GRC	Glass Reinforced Plastic
DI	Ductile Iron
PVC	Polyvinyl Chloride
CI	Cast Iron
SI	Spun Iron
ST	Steel
VC	Vitrified Clay
PP	Polypropylene
PF	Pitch Fibre
MAC	Masonry, Coursed
MAR	Masonry, Random
U	Unspecified

Address or Site Reference:

1 WATERS EDGE CLOSE,
WHITEHAVEN,
CA28 9PE

OS sheet NX9616SE

Number:

Date: 01/07/2021

Scale: 1:1250

Nodes: 401

Sheet: 4 of 6

Printed by: Property Searches

Reho	Cover	Func	Invert	Size x	Size y	Shape	Matl	Length	Grad
8128	93.5	FO	93.3	150			PVC	13.54145	1 in 16
8301	91.55	SW	90.3	300			VC	72.59887	1 in 70
8129	94.93	FO	92.75	150			PVC	14.65986	1 in 163
8019		FO		100			PVC	5.297863	
7409		CO	0	100			VC	3.016879	
7409		CO	0	100			VC	3.016879	
5402	83.4	SW	78.97	2400			PE	44.07374	1 in 67
5402	83.4	SW	78.97	2400			PE	44.07374	1 in 67
9431	92	CO	90.71	150			VC	3.605551	
9431	92	CO	90.71	150			VC	3.605551	
8515	93.28	CO	92.7	100			VC	10.8033	1 in 54
8515	93.28	CO	92.5	100			VC	10.8033	1 in 54
9513	89.91	CO	89.3	100			VC	24.35159	
9513	89.91	CO	89.3	100			VC	24.35159	
9202		CO		100			VC	2.236966	
6413	90.97	FO	88.9	150			VC	17.31798	1 in 13
9417	95.33	CO	94.17	100			VC	28.16026	
8003	97.54	SW	94.23	150			VC	47.30701	1 in 99
9517	90.56	CO	89.76	150			VC	19.23338	
9517	90.56	CO	89.76	150			VC	19.23338	
9206		CO	0	100			VC	7.526254	
8231		CO		100			VC	23.34524	
6414	85.1	FO	83.16	150			VC	77.02306	1 in 20
6414	85.1	FO	83.16	150			VC	77.02306	1 in 20
6415	85.2	SW	83.6	375			CO	34.7841	1 in 11
6415	85.2	SW	83.6	375			CO	34.7841	1 in 11
6410	89.14	FO	87.15	150			VC	12.2688	1 in 11
6410	89.14	FO	87.15	150			VC	12.2688	1 in 11
8004	86.2	SW	83.13	450			PE	4.915501	1 in 15
8326	92.78	CO	91.13	150			VC	6.082763	
4402	79.09	SW	77.8	225			VC	97.17252	1 in 32
4402	79.09	SW	77.8	225			VC	97.17252	1 in 32
4402	79.09	SW	77.8	225			VC	97.17252	1 in 32
4402	79.09	SW	77.8	225			VC	97.17252	1 in 32
9907	97.05	SW	94.29	375			CO	21.54376	1 in 268
8402	93.26	CO	91.84	225			VC	97.36134	1 in 78
8402	93.26	CO	91.84	225			VC	97.36134	1 in 78
8133	94.94	CO	93.1	225			VC	2.633412	1 in 8
9134	96.28	CO	93.78	150			VC	13.95882	1 in 41
7410		CO	0	150			VC	27.313	
7410		CO	0	150			VC	27.313	
8229		CO		225			VC	34.34416	
8105	95.3	SW	92.94	225			VC	58.55811	1 in 101
5402	83.4	SW	78.97	2400			PE	37.45959	1 in 57
5402	83.4	SW	78.97	2400			PE	37.45959	1 in 57
8125	94.94	SW	92.73	600			CO	11.64635	1 in 233
8235		CO		100			VC	14.40156	
9033	97.86	CO	96.86	100			VC	3.162278	1 in 45
8206		CO		100			VC	25.91596	
8201		FO		150			PVC	21.93114	
5403	81.8	SW	78.3	225			VC	38.33781	1 in 167
5403	81.8	SW	78.3	225			VC	38.33781	1 in 167
5403	81.8	SW	78.3	225			VC	38.33781	1 in 167
5403	81.8	SW	78.3	225			VC	38.33781	1 in 167
6305	91.58	FO	89.92	150			VC	67.5574	1 in 66
9104	95.91	CO	93.95	150			VC	8.014686	1 in 47
5406	79.4	FO	78.09	225			VC	34.37752	1 in 40
5406	79.4	FO	78.09	225			VC	34.37752	1 in 40
5406	79.4	FO	78.09	225			VC	34.37752	1 in 40
8306	92.72	CO	90.35	375			VC	89.41511	1 in 115
7418	92.77	SW	91.43	150			VC	15.80049	1 in 17
9121	98.99	CO	97.21	150			VC	63.01686	
6407	90.88	SW	89.26	300			VC	17.51682	1 in 14
9123	97.19	CO	95.68	100			VC	2	
5407	82	SW	78.31	450			PE	3.328226	1 in 333
5407	82	SW	78.31	450			PE	3.328226	1 in 333
5407	82	SW	78.31	450			PE	3.328226	1 in 333
5407	82	SW	78.31	450			PE	3.328226	1 in 333
0909		CO	0	100			VC	4.14843	
8126	94.8	SW	0				CO	0.6266254	
4301	81.81	FO	78.88	225			VC	80.06976	1 in 102
4301	81.81	FO	78.88	225			VC	80.06976	1 in 102
6403	88.58	SW	85.81	375			CO	54.91792	1 in 25
6403	88.58	SW	85.81	375			CO	54.91792	1 in 25
9911	97.08	FO	95.07	150			PVC	25.26822	1 in 149
9411		CO	0	225			VC	51.05484	
9411		CO	0	225			VC	51.05484	
9929	96.46	SW	94.46	225			PVC	16.68862	1 in 70
8131	94.93	FO	93.03	150			PVC	22.01189	1 in 147
4501	76.25	SW	74.74	225			VC	99.18073	1 in 22
4501	76.25	SW	74.74	225			VC	99.18073	1 in 22
4501	76.25	SW	74.74	225			VC	99.18073	1 in 22
4501	76.25	SW	74.74	225			VC	99.18073	1 in 22
8241		CO		100			VC	10.46606	
8534		CO		100			VC	8.598037	
8534		CO		100			VC	8.598037	
9030		CO	0	100			VC	11.83307	
9414	93.5	CO	92.5	100			VC	12.84911	
9414	93.5	CO	92.5	100			VC	12.84911	
8505	91.46	CO	90.95	150			VC	19.41849	
7409		CO	0	100			VC	6.202866	
7409		CO	0	100			VC	6.202866	
0419	90.82	CO		100			VC	15.85093	
0419	90.82	CO		100			VC	15.85093	
9115		CO	0	100			VC	8.062258	
4201	83.39	FO	79.89	225			VC	87.37727	1 in 99
4201	83.39	FO	79.89	225			VC	87.37727	1 in 99
8238		CO		100			VC	4.506834	
7414		CO		100			VC	4.135752	
7414		CO		100			VC	4.135752	
8405	93.24	CO		100			VC	2.044396	
8405	93.24	CO		100			VC	2.044396	
8130	94.79	FO	92.88	150			PVC	16.64019	1 in 128
7214	93.34	CO	92.18	100			VC	16.93122	
9042	97.08	FO	94.49	150			PVC	31.4377	1 in 150
8108	94.94	CO	92.41	150			VC	10.88622	1 in 156
8110		FO	92	150			PVC	5.076265	
8331	93.37	CO	92	150			VC	17.02939	
9440		CO	89.77	100			VC	8.190701	
9440		CO	89.77	100			VC	8.190701	
9401	91.87	SW	90.43	225			VC	47.5964	1 in 17
9909	96.61	SW	94.17	375			CO	58.24958	1 in 121
5401	78.37	FO	77.21	225			VC	23.55125	
5401	78.37	FO	77.21	225			VC	23.55125	
5401	78.37	FO	77.21	225			VC	23.55125	
5401	78.37	FO	77.21	225			VC	23.55125	
8132	94.92	CO	92.86	150			PVC	2.833179	1 in 11
9312		CO		100			VC	69.2315	
8127	93.63	FO	93.63	150			PVC	6.983881	1 in 233
9901		CO	0	225			VC	51.10773	
6406	89.49	SW	87.97	300			VC	9.91241	1 in 22
6406	89.49	SW	87.97	300			VC	9.91241	1 in 22
9514	90.21	CO	89.56	100			VC	14.42235	1 in 55
9514	90.21	CO	89.56	100			VC	14.42235	1 in 55
8123	94.54	SW	92.85	600			CO	22.19487	1 in 317
8535		CO		100			VC	4.298528	
8535		CO		100			VC	4.298528	
7211		CO	0	100			VC	17.02943	
6002	96.89	SW	93.75	150			VC	10.44419	1 in 33

Reho	Cover	Func	Invert	Size x	Size y	Shape	Matl	Length	Grad
------	-------	------	--------	--------	--------	-------	------	--------	------

Abandoned

Foul

Surface Water

Combined

Public Sewer

Private Sewer

Section 104

Rising Main

Sludge Main

Overflow

Water Course

Highway Drain

All point assets follow the standard colour convention:

red - combined

blue - surface water

brown - foul

purple - overflow

Manhole

Head of System

Extent of Survey

Rodding Eye

Inlet

Discharge Point

Vortex

Penstock

Washout Chamber

Valve

Air Valve

Non Return Valve

Soakaway

Gully

Cascade

Flow Meter

Hatch Box

Oil Interceptor

Summit

Drop Shaft

Orifice Plate

Side Entry Manhole

Outfall

Screen Chamber

Inspection Chamber

Bifurcation Chamber

Lamp Hole

T Junction / Saddle

Catchpit

Valve Chamber

Vent Column

Vortex Chamber

Penstock Chamber

Network Storage Tank

Sewer Overflow

Ww Treatment Works

Septic Tank

Control Kiosk

Change of Characteristic

MANHOLE FUNCTION

FO Foul

SW Surface Water

CO Combined

OV Overflow

SEWER SHAPE

CI Circular

EG Egg

OV Oval

FT Flat Top

RE Rectangular

SQ Square

TR Trapezoidal

AR Arch

BA Barrel

HO HorseShoe

UN Unspecified

SEWER MATERIAL

AC Asbestos Cement

BR Brick

PE Polyethylene

RP Reinforced Plastic Matrix

CO Concrete

CSB Concrete Segment Bolted

CSU Concrete Segment Unbolted

CC Concrete Box Culverted

PSC Plastic / Steel Composite

GRC Glass Reinforced Plastic

DI Ductile Iron

PVC Polyvinyl Chloride

CI Cast Iron

SI Spun Iron

ST Steel

VC Vitrified Clay

PP Polypropylene

PF Pitch Fibre

MAC Masonry, Coursed

MAR Masonry, Random

U Unspecified

Address or Site Reference:

1 WATERS EDGE CLOSE,
WHITEHAVEN,
CA28 9PE

OS sheet
Number:
Scale: 1:1250

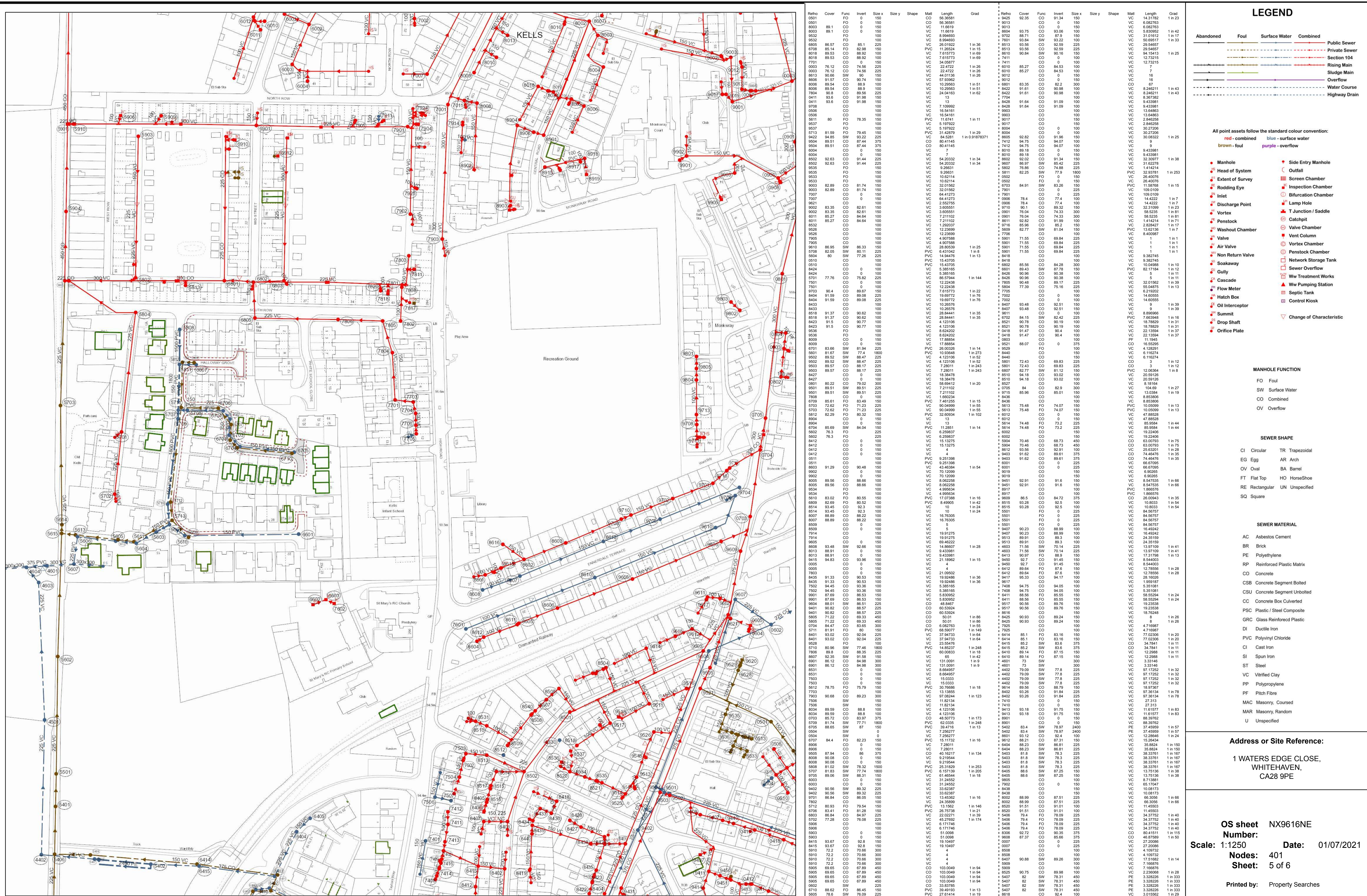
NX9616SE
Date: 01/07/2021
Nodes: 130
Sheet: 4 of 6

Printed by: Property Searches

SEWER RECORDS

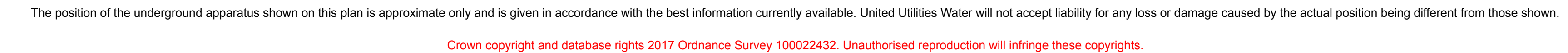
United Utilities

Water for the North West



The position of the underground apparatus shown on this plan is approximate only and is given in accordance with the best information currently available. United Utilities Water will not accept liability for any loss or damage caused by the actual position being different from those shown.

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SEWER RECORDS  **United Utilities**

APPENDIX D – PREVIOUS DRAINAGE INVESTIGATIONS

Former Rhodia Site Sea Outfall Investigation

Week Commencing 11th December 2017

Purpose: To gather as much information as possible about the sea outfall and its associated underground physical assets upstream to the leachate plant sampling point.

The sea outfall was installed to discharge all the sites' effluent into the Irish Sea. In the late 1990's the outfall discharged up to 40 megalitres per day. After the site closed and the buildings were demolished the only effluent stream remaining is the settled leachate that is pumped to sea via the sea outfall.

The environment discharge permit for the leachate system dated 2007 (but believed to originate in the early 2000's) describes the leachate being discharged through a 600mm diameter drain from a sample point -designated S01 at grid reference NX9649 1583 and then to the sea outfall with a discharge point given at the low water mark grid reference NX9602 1627.

There are two separate sea outfall pipes; one that is currently used for the leachate outfall; this starts at MH566 upstream of S01 and leads down to the cliff edge and the consented discharge point below MH1. There is a second pipe that begins at MH9 on site and runs via MH17, MH15, 14, 13, 12 & 11 to the second sea outfall. We have inspected most of this and found it to be a serviceable condition. There's no access to the vertical section.

We started our investigation on the leachate discharge system at the two manholes upstream of S01 – these are marked on the drawing as S01A and MH566. Manhole 566 has a 225mm invert that rapidly opens to a 600mm drain that leads to S01A. At S01A the drain reduces to 150mm diameter. It flows to the sample point S01 in a 150mm diameter plastic pipe. After S01 the pipe reduces to 100mm diameter.

We next find the plastic pipe in manhole 10, grid reference NX9641 1598; here the pipe is sitting in the invert of a 600mm drain; we think this is the original drain as referenced in the discharge permit. Having the plastic pipe in the invert makes it impossible to obtain travelling CCTV footage so we have not managed to confirm the condition of the host drain along its length. What we have seen indicates that it is fair condition.

There is a manhole upstream of MH10, it's a 450mm lateral connection to MH10 and it is designated MH300 on the plan, it's grid reference is NX9644 1598. We have inspected from it down to MH10 and find the drain to be in good condition. MH300 could be a useful manhole as it would give access to the leachate sea outfall for any pumped water from the old anhydrite mine.

Going downstream from MH10 the next available manhole is MH7 at grid reference NX9635 1613. This is outside the site boundary. The plastic pipe is visible in the invert. From here the plastic pipe is next seen in the flume and dosing chamber -we have called this MH17C. This is a locked and welded compound with no access; it's grid reference is NX9634 1614. From here the host drain and the plastic pipe are found at MH4, MH3 & MH2 and it is believed to enter the sea outfall at MH1. MH1 is

another locked and welded structure. At MH4 the pipe is visible in the invert, as it is at MH3. We managed to push a camera from MH3 down to MH2; the host pipe appears intact and in fair condition. At MH2 there is a dropshaft down to MH1. The plastic pipe traverses the dropshaft and exits the manhole through a wall. The pipe is anchored through this wall. MH2 is a dog leg manhole with a large cast concrete lid. There's no access from above. There's also a diverter drain running from MH2 across to MH12; there's no flow down this pipe.

There is an unmarked 225mm drain flowing from a southerly direction through a manhole designated MH3A. This is a light duty cover with a constant 20% flow that discharges into MH3. This flows around the plastic pipe to MH2 and then into the dropshaft. Dye tests of the leachate drain indicated that leachate was not flowing through this unmarked drain.

As there is no access at MH1 we could not determine the internal condition of the vertical discharge.

Video footage of the Greenpeace activities on this outfall in 1991 shows a swan's neck outfall, with the top of the neck at the high-water mark. This swan's neck has been washed away in the intervening period, the horizontal section to the low water mark has been open to the elements for several years.

There is a second sea outfall pipe; this runs down the cliff from MH17, through the southern side of MH17C to MH15 which is a Y manhole. This connects across to MH5. The other leg of the Y is stopped off but it runs down to MH14 then to MH13, then to MH12 and finally it drops into a box section culvert called MH11 on the plan before discharging over the cliff. We have footage of this drain – it's a combination of 600 and 450mm diameter pipe and is in good condition. Access to the vertical section is poor and unsafe. There's no access at MH11 other than by dangling over the cliff.

Upstream of MH17 the drain flows from MH9 – an on-site manhole with grid reference NX9638 1597. Manhole 9 has a cast slab cover – very similar to MH2 in this respect and it would need heavy lifting gear to open. We did get the camera upstream to MH9 only to find a half wall blocking the pipe. Water is flowing over this weir wall.

When you look at the site drawing there's an octagonal base to the east of MH9. This used to be an effluent "treatment" unit and I believe it connected to MH9 via a 600mm drain. We could not find any access points for this drain.

Conclusions

We have not been able to confirm the condition of the vertical section or the low water horizontal section as there is currently no access to these parts of the pipe. Unlocking MH1 and undertaking a confined space entry will be required.

There is an existing route for discharges of pumped water. The leachate discharge system has been modified, compared to the consent, by the installation of a 110mm continuously welded plastic pipe from sample point S01 down to the sea discharge. We presume this was done to comply with the consent to discharge 017490484 "condition 2 – nature of the discharge. The discharge shall consist solely of trade effluent...." However, the cross connection from MH3A to MH3 implies that this condition is currently in breach. The 110mm diameter pipe is probably insufficient for the planned mine water discharge.

The plastic pipe is laid, so far as we can tell, as a loose fit in the existing pipe. As such, it may be possible to utilise the host pipe to carry the mine water to sea. The discharge pipe had a working capacity of 40 megalitres per day when in operation.

The condition of the pipe is generally fair along the lengths we have managed to inspect.

There is a second route from the site via MH 9, 17, 15,14,13,12,11 to the second sea outfall. There's one section of 450mm pipe, all the rest, apart from the final box section is 600mm. The report describes the condition of the drain which is generally acceptable; however, we could not investigate the vertical drop to the low water mark. Further investigation would need to be made to inspect MH9.

Ian Woodhouse

Director

Unblock Cumbria Ltd

West Cumbria Mining
Data

Former Rhodia Site Discharge Pipeline

Lat WGS84	Long WGS84	Point Name	Grid Ref OSGB36	Height CL (m)	Depth IL (m)	Diameter	Comments
54.31'47.22135	-3.36'11.24454	0001/MH17	NX9635 1612	135.22	128.62		
54.31'47.495375	-3.36'11'14871	0002/MH7	NX9635 1613	132.321	127.501		
54.31'47'74426	-3.36'11.60059	0003/MH17 C	NX9634 1614	130.257	Not Available		See Note 1
54.31'47.90225	-3.36'14.41532	0004/MH15	NX9635 1614	124.988	122.608		
54.31'48.18553	-3.36'14.84566	0005/MH5	NX9628 1615	123.777	120.507		
54.31'48.67863	-3.36'20.78765	0006/MH3A	NX9618 1617	107.49	106.97		
54.31'49.02763	-3.36'21.68920	0007/MH13	NX9619 1618	105.167	103.397		
54.31'48.87476	-3.36'22.72649	0008/MH3	NX9614 1618	103.036	101.116		
54.31'49.13679	-3.36'22.54640	0009/MH2	NX9615 1619	102.436	Not Available		Concrete slab
54.31'48.91533	-3.36'23.33640	0010/MH12	NX9613 1618	101.607	99.997		
54.31'50.16636	-3.36'24.01693	0011/MH1	NX9612 1622	95.73	94.63		See Note 2
54.31'49.39749	-3.36'27.06512	0012/MH11	NX9607 1620	85.034	Not Available		See Note 3
54.31'48.29819	-3.36'18.15023	0013/MH14	NX9623 1616	113.693	111.473		
54.31'48.59459	-3.36'17.81828	0014/MH4	NX9623 1617	113.93	112.03		
54.31'35.62159	-3.36'0332579	0015/MH566	NX9648 1576	138.625	136.805		
54.31'37.39589	-3.36'02.76644	0016/MHS01A	NX9649 1582	138.412	135.912		
54.31'37.71606	-3.36'02.91861	0017/MHS01	NX9649 1583	138.004	135.074		
54.31'42.75301	-3.36'06.07332	0018/MH300	NX9644 1598	138.26	136.26		
54.31'42.79022	-3.36'07.71437	0019/MH10	NX9641 1598	138.284	132.634		
54.31'45.22282	-3.36'09.35327	0020/MH9	NX9638 1597	137.365	Not Available		Concrete Slab

Note Heights are expressed in metres above Wgs84 ellipsoid surface. Invert levels are measured and deducted from cover levels

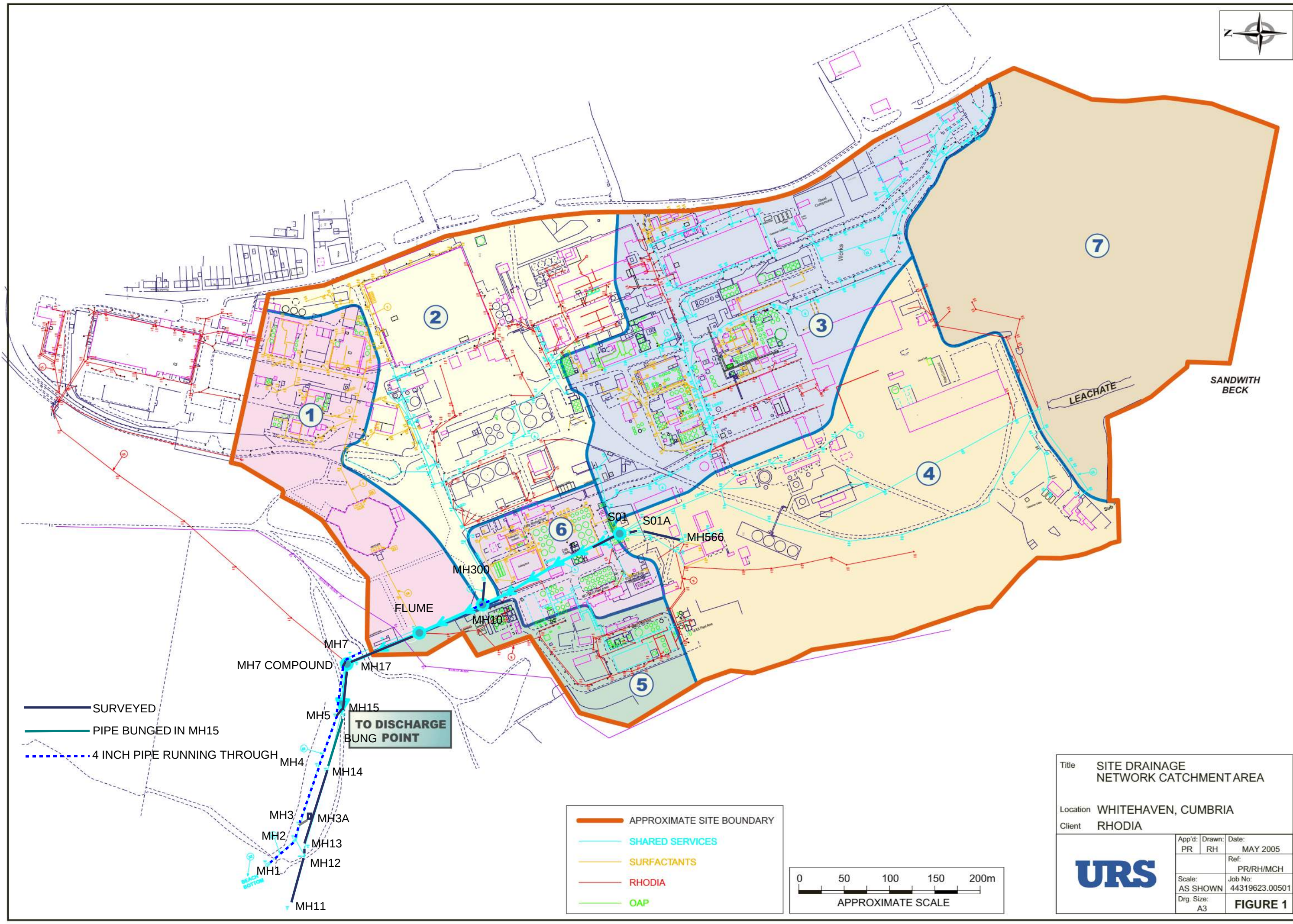
Note 1 MH17C is the unmarked compound that is locked and welded

Note 2 MH1 is the locked & welded vertical drop on the cliff edge. IL is measured to the top of the discharge pipe

Note 3 MH11 is the other sea discharge pipe. The cover level is the top of the flume - alongside the concrete structure

Manhole Photos From Upstream MH 566 to the Sea Outfall below MH1





APPENDIX E– EXISTING OUTFALL PHOTOGRAPHS



Photo 1 – Northern Discharge Pipe – Shaft (I)



Photo 2 – Northern Discharge Pipe – Shaft (II)

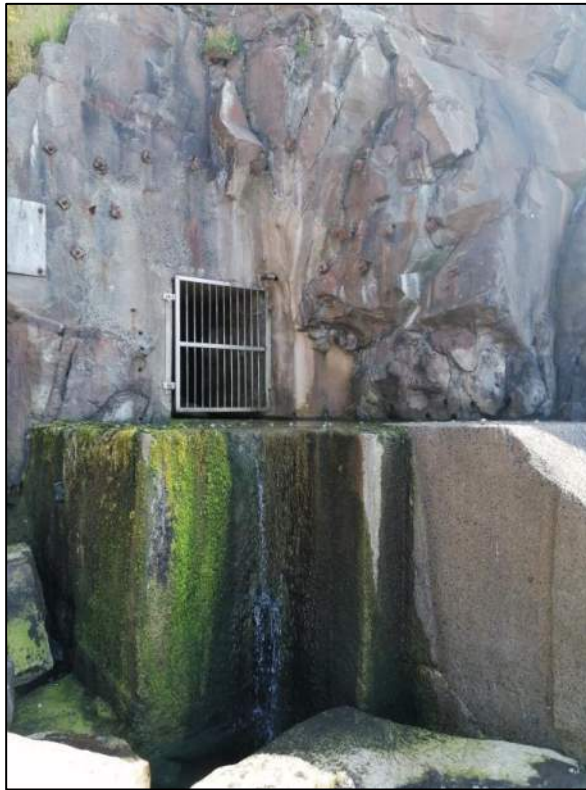


Photo 3 – Northern Discharge Pipe – Sea Outfall (I)



Photo 4 – Northern Discharge Pipe – Sea Outfall (II)

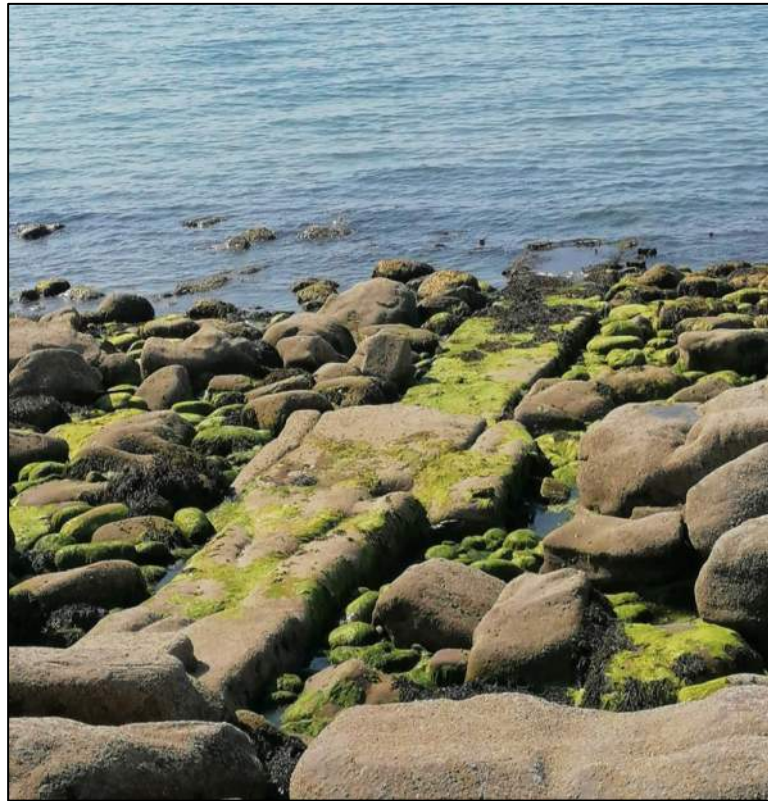


Photo 5 – Northern Discharge Pipe – Sea Outfall (III)



Photo 6 – Cliff Outfall (I)



Photo 7 – Cliff Outfall (II)

APPENDIX F – GREENFIELD RUNOFF CALCULATIONS

Calculated by:	Francisco Aguilar
Site name:	marchon
Site location:	

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance "Rainfall runoff management for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Site Details

Latitude:	54.52880° N
Longitude:	3.59867° W
Reference:	3768447705
Date:	Jul 01 2021 11:08

Runoff estimation approach

IH124

Site characteristics

Total site area (ha):	28.34
-----------------------	-------

Methodology

Q _{BAR} estimation method:	Calculate from SPR and SAAR
SPR estimation method:	Calculate from SOIL type

Soil characteristics

	Default	Edited
SOIL type:	4	4
HOST class:	N/A	N/A
SPR/SPRHOST:	0.47	0.47

Hydrological characteristics

	Default	Edited
SAAR (mm):	1058	1058
Hydrological region:	10	10
Growth curve factor 1 year:	0.87	0.87
Growth curve factor 30 years:	1.7	1.7
Growth curve factor 100 years:	2.08	2.08
Growth curve factor 200 years:	2.37	2.37

Notes**(1) Is $Q_{BAR} < 2.0$ l/s/ha?**

When Q_{BAR} is < 2.0 l/s/ha then limiting discharge rates are set at 2.0 l/s/ha.

(2) Are flow rates < 5.0 l/s?

Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage from vegetation and other materials is possible. Lower consent flow rates may be set where the blockage risk is addressed by using appropriate drainage elements.

(3) Is $SPR/SPRHOST \leq 0.3$?

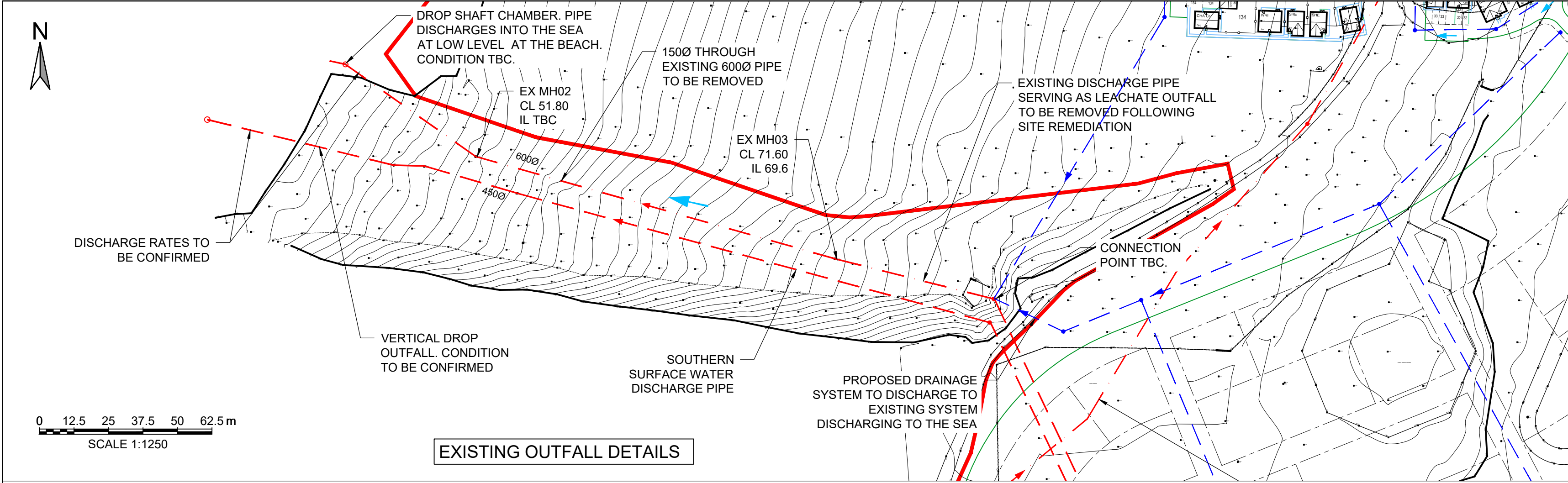
Where groundwater levels are low enough the use of soakaways to avoid discharge offsite would normally be preferred for disposal of surface water runoff.

Greenfield runoff rates

	Default	Edited
Q _{BAR} (l/s):	221.84	221.84
1 in 1 year (l/s):	193	193
1 in 30 years (l/s):	377.13	377.13
1 in 100 year (l/s):	461.42	461.42
1 in 200 years (l/s):	525.76	525.76

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.

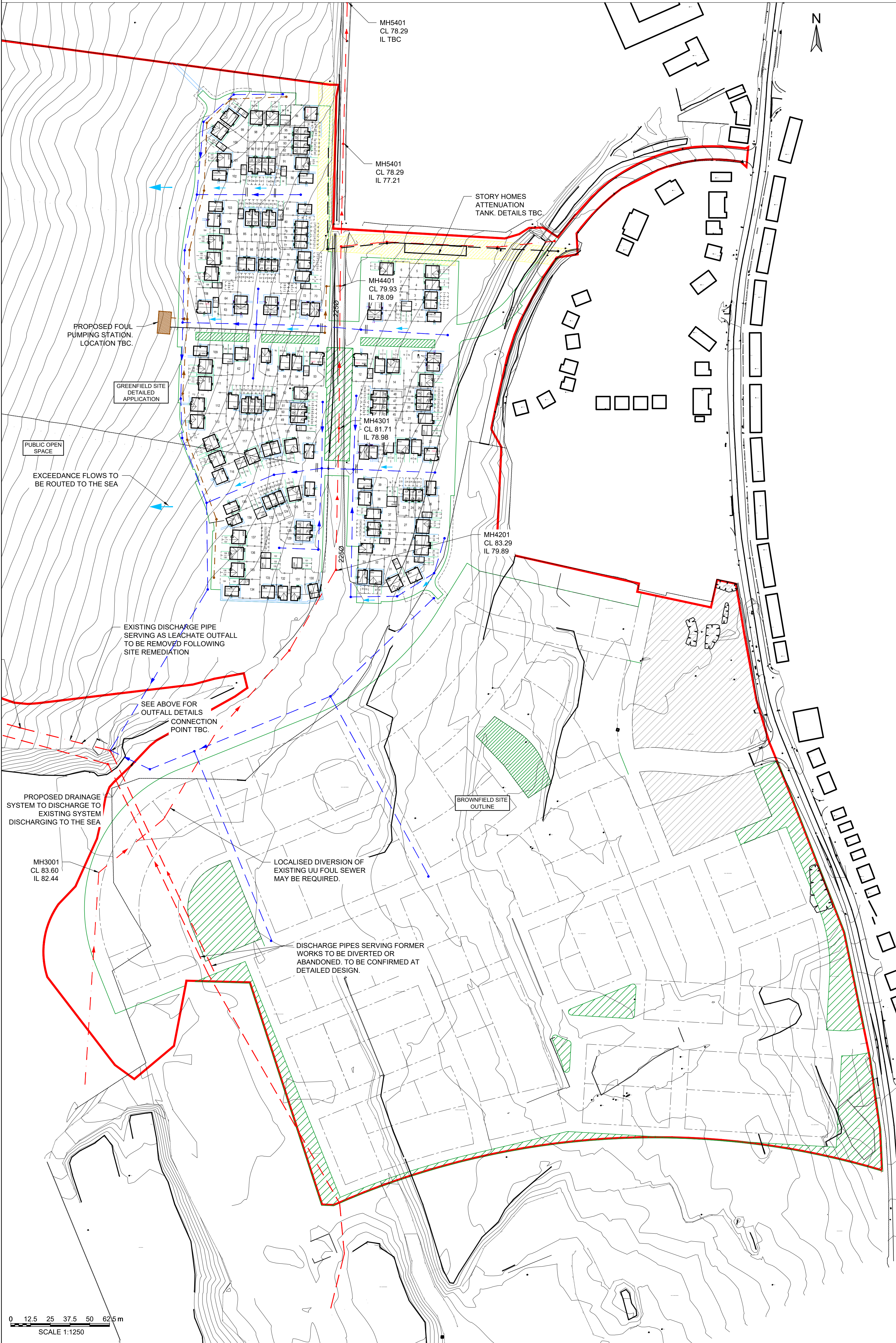
APPENDIX G – PROPOSED DRAINAGE LAYOUT



- NOTES
1. ALL LEVELS IN METRES AOD.
 2. SEWER RECORDS BY UNITED UTILITIES.
 3. WATERS EDGE CLOSE DRAINAGE RECORDS PROVIDED BY STORY HOMES.
 4. DRAINAGE DETAILS FROM FORMER MARCHON SITE BASED ON REPORT BY UNBLOCK CUMBRIA DATED DECEMBER 2017.
 5. MASTERPLAN BY CONCEPT ARCHITECTURE PROJECT NO. CPT-257 DRAWING NO. FEA-02 DATED 08.03.2021
 6. TOPOGRAPHICAL SURVEY BY RJP SURVEYING CONSULTANTS LTD DRAWING PH376/T00 DATED JULY 2018

- KEY
- EXISTING FOUL PUBLIC SEWER
 - EXISTING SURFACE WATER SEWER
 - EXISTING SURFACE WATER DRAINAGE
 - PROPOSED FOUL DRAINAGE
 - PROPOSED RISING MAIN
 - PROPOSED SURFACE WATER DRAINAGE
 - EXCEEDANCE OVERLAND FLOW ROUTE

THIS DRAWING IS FOR PRELIMINARY DESIGN PURPOSES ONLY AND SUBJECT TO FURTHER DETAILED DESIGN



PRELIMINARY ISSUE

P02	PRELIMINARY PLANNING ISSUE	10.08.2021	FA	CD	CD
P01	PRELIMINARY FIRST ISSUE	01.07.2021	FA		
Rev	Description	Date	Drn / Chk / App		

Issuing Office
Tetra Tech Leeds
3 Sovereign Square, Sovereign Street,
Leeds, United Kingdom, LS1 4ER
Tel: +44 (0)11 3278 7111
www.tetrateteurope.com



Client
PERSIMMON HOMES

Project Name
**PROPOSED RESIDENTIAL DEVELOPMENT,
FORMER MARCHON SITE,
WHITEHAVEN**

Sheet Title
INDICATIVE DRAINAGE LAYOUT

TTE Project Number	Drawn By	Date	Checked By	Date	Approved By	Date	Scale @ A1	Suitability
B030491	FA	JULY 21	CD	AUG 21	CD	AUG 21	1:1250	S0
Client Project Number	Originator	Volume/System Level/Location	Type/Code	Role	Number	Revision		
B030491	- TTE	- 00	- XX	- DR	- CH	- SK001	P02	